

Analysis of Pedernales River Water Quality

Report to the Lower Colorado River Authority

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Executive Summary

The LCRA water quality database for the Pedernales River was analyzed to determine current conditions and whether there are significant water quality trends. Possible trends include spatial trends, in which the concentrations vary along the length of the river, and temporal trends, meaning a variation in concentrations through time at a single monitoring site.

The data were collected by the LCRA and TNRCC at four sites along the Pedernales River: RR 1320, Johnson City, RM 962, and upstream of the confluence with Falls Creek (the lower boundary of this river segment). Although the length of the monitoring period varied among the sites, the majority of the data were collected between 1984 and 1997. During this period about 7200 measurements of 24 constituents were made at the monitoring sites. A subset of these constituents was selected for detailed statistical analysis. The selected constituents include measures of oxygen content, dissolved solids, nutrients and bacteria. The constituents analyzed include:

- Dissolved Oxygen
- Specific Conductance
- Sulfate
- Chloride
- Ammonia
- Nitrate + Nitrite
- Total Kjeldahl Nitrogen
- Total Organic Carbon
- Total Phosphorus
- Fecal Coliform

Analysis of the LCRA water quality database for the Pedernales River indicates that several constituents exhibit a significant spatial trend. All of the constituents that varied

had higher concentrations upstream than down. These constituents are dissolved oxygen, chloride, sulfate, nitrate plus nitrite, and specific conductance (a measure of dissolved solids). The monitoring data are not sufficient for identifying the reasons for these changes, which may be natural, manmade, or a combination of the two.

The higher levels of dissolved solids, nitrogen, and other ionic species may be derived from groundwater inflow to the river. Groundwater often contains higher concentrations of these constituents than surface water. Elevated concentrations of these constituents also are associated with agricultural activities. Irrigation return flows have higher levels of dissolved solids and can carry nutrients derived from fertilizers as well as agricultural pesticides. Measurements of pesticide concentrations were not included in the monitoring program. Determination of the causes of the higher concentrations should be the focus of future studies, because dissolved solids, chloride, and sulfate are all listed as parameters of possible concern in the 1994 LCRA water quality assessment (LCRA, 1994). Concentrations of these constituents are currently below levels that caused them to be flagged in the 1994 report.

Temporal trends were most evident at the upstream monitoring station at RR 1320, which generally had the longest period of record. Constituents with significant temporal trends include specific conductance, chloride, and sulfate. The concentrations of each of these showed a reduction with time. The monitoring site at RM 962 showed a single parameter with a temporal trend, chloride, which also had declining concentrations. There was no significant temporal trend for any constituent at Johnson City or the Falls Creek sites. These reductions in concentration may be the result of much higher than average rainfall in 1991 and 1992. Rainfall generally has very low concentrations of these constituents, causing a dilution in groundwater, which contributes baseflow to the river, as well as in the river itself through increased surface runoff.

The water quality of the Pedernales River can be characterized as very good and supportive of all designated beneficial uses. Current concentrations of all major water quality constituents are generally below levels that would be cause for concern or result

in the listing of this river segment as impaired. In addition, this data set indicates that the concentrations of the analyzed constituents at each monitoring site are relatively constant or improving. There is no immediate concern that changes in land use or other human activities threaten the water quality of the Pedernales River; however, the source of higher concentrations of dissolved solids and nutrients in the upstream reach of the river should be identified. The change from undeveloped to agricultural land use occurred many years before the beginning of any water quality monitoring programs and may be responsible for higher constituent concentrations in this portion of the watershed.

Despite the generally high quality of the Pedernales River water, eight fish kill episodes have been reported in the river, including five since 1990. Approximately half of the kills have been the result of illegal dumping of toxic substances. These episodes highlight the importance of an effective public education/outreach program. Such a program can make citizens more aware of the environmental impacts of improper disposal of waste materials. It is especially important to target owners of small businesses, which often need to dispose of significant quantities of spent solvents, lubricants, paint and other toxic materials.

The remaining fish kills were the result of wastewater treatment plant (WWTP) discharges to the Pedernales near Johnson City, resulting in low dissolved oxygen concentrations. Conventional water quality monitoring programs are not effective for identifying episodic events resulting from equipment malfunction or other causes; however, wastewater discharges may be responsible for the trend of decreasing dissolved oxygen concentrations from upstream to downstream. The number of fish kills related to wastewater discharges suggests that a review of the permit requirements and adequacy of the Johnson City WWTP should be a high priority.

Introduction

The LCRA has been collecting periodic water quality data from a number of sites along the Colorado River and its tributaries since the early 1980's. Geographically, the sites range from upstream of San Saba to Matagorda Bay. One of the main functions of data collection is to detect long term changes in water quality that could indicate possible causes and provide a quantitative basis for management actions. Changes in water quality may reflect the impacts of urbanization or other changes in land use patterns.

Because of the size and complexity of the data analysis required to determine current conditions and trends, a pilot study was determined to be the best way to evaluate the potential problems and time required to complete a study of the entire lower Colorado River watershed. The Pedernales River was selected as the site for initial analysis, because it was deemed representative of river system analysis, while having a much smaller data set for analysis.

The Pedernales also has been the site of five reported fish kills since 1990 that were caused primarily by illegal dumping or by wastewater treatment plant discharges. However, a general decline in water quality of this segment could have contributed to these episodes and this is an additional reason to select the Pedernales River for analysis of water quality trends.

Scope

Sufficient water quality data for analyzing temporal and spatial trends of water quality in the Pedernales River have been collected at four sites. From upstream to downstream, these sites are RR 1320 (Site 150), Johnson City (Site 75), RM 962 (Site 25), and just upstream of the confluence with Falls Creek (Site 15). Samples were collected between 1984 and 1997 by the LCRA and the Texas Natural Resource Conservation Commission, although the period of record varies for individual constituents. The data allow the determination of water quality changes that have occurred since 1984.

There are two primary questions addressed in this pilot study. First, does the data exhibit discernible trends through time within the given subwatershed? Second, does the data exhibit discernible trends through space along the length of the river? This study answers these questions and provides a template for further analysis of water quality data for the LCRA service area.

The constituents analyzed include:

- Dissolved Oxygen
- Specific Conductance
- Sulfate
- Chloride
- Ammonia
- Nitrate + Nitrite
- Total Kjeldahl Nitrogen
- Total Organic Carbon
- Total Phosphorus
- Fecal Coliform

Methodology

There are numerous statistical methods for analyzing temporal and spatial trends in water quality data and their appropriateness often depends on the underlying distribution of the data. Many analyses (parametric tests) assume that the data are normally distributed; however, environmental data often does not follow this distribution. Nonparametric statistical tests are used when the distribution of the data is unknown and although they may not yield as much information, they are more robust. Consequently, many of the analyses were done with both tests, where appropriate, in order to yield the strongest results possible.

Flow rate of the Pedernales was not recorded at each location at the time of sampling. Since this is such an important parameter, average daily flow at the U.S. Geological Survey gauge at Johnson City was used to characterize conditions at the time of sampling. Although the absolute value is only correct for the Johnson City monitoring site, the numbers offer a quantitative way to differentiate between high and low flow-sampling conditions.

Many of the measurements are censored; that is, they are reported as less than or greater than some value associated with the detection limit of the method. There are numerous methods for treating censored data by assuming arbitrary values (the detection limit or zero, for instance) or by identifying the underlying distribution of the data and estimating the true value. It is far better to perform statistical analyses with the actual measured concentrations even though they fall below the instrument or method detection limit; however, laboratories rarely report results in this form. For this analysis, all censored values were assumed to have the same concentration as the detection limit.

Temporal Trends

Regression analyses are commonly used to identify temporal trends in water quality data. If plots of data versus time suggest a simple linear increase or decrease over time, a linear regression of the variable against time may be fit to the data. A *t* test may be used to test

that the true slope is not different from zero. This t test can be misleading if seasonal cycles are present, the data are not normally distributed, and/or the data are serially correlated. In these situations, the t test may indicate a significant slope when the true slope actually is zero (Gilbert, 1987).

Multiple linear regression is an especially useful approach if other variables such as flow rate or temperature also affect parameter values. Since flow rate is such an important factor, especially as it relates to storm runoff and baseflow, the data at each site was first analyzed for temporal trends using multiple linear regression. The statistical software package, *Minitab for Windows: Release 12*, was used for these analyses.

For the constituents that appeared to exhibit a significant temporal trend in the multiple linear regression tests, the data also were analyzed using a nonparametric test, the Mann-Kendall test. This procedure is particularly useful since missing values are allowed and the data need not conform to any particular distribution. Also, data reported as trace or less than the detection limit can be used by assigning them a common value that is smaller than the smallest measured value in the data set. The Mann-Kendall test can be viewed as a nonparametric test for zero slope of the linear regression (Gilbert, 1987). The test consists of a comparison of the difference of all possible pairs of values and indicates whether the values are generally increasing or decreasing, but gives no indication of the rate of change. This test is not commonly available in commercial software packages, so it was implemented as a spreadsheet.

Spatial Trends

Analysis of variance (ANOVA) was the statistical technique used to determine whether there are spatial trends in the water quality data (e.g., whether the mean concentrations at the sites are different). ANOVA is similar to regression in that it is used to investigate and model the relationship between a response variable and one or more independent variables. However, analysis of variance differs from regression in two ways: the independent variables are qualitative (sampling location), and no assumption is made about the nature of the relationship (i.e. the model does not include coefficients for

variables). In effect, analysis of variance extends the two-sample t-test for testing the equality of two population means to a more general null hypothesis of comparing the equality of more than two means, versus them not all being equal. *Minitab for Windows: Release 12* was used for the ANOVA calculations.

For selected constituents, the data were corrected for changes in sample collection time or flow rate and then the ANOVA analysis was performed. This correction consisted of normalizing the data at each site to a consistent time or flow basis. This was accomplished by performing a linear regression against the influencing variable and calculating the average value for an arbitrary time or discharge rate. The residual error for each measurement was then added to the average value and the ANOVA test was then performed on the new data set.

Results

Dissolved Oxygen

Summary Statistics

The analysis of temporal and spatial trends in dissolved oxygen concentrations is especially difficult because of the number of factors that affect this parameter. Of particular importance is temperature, depth of sample and time of day. Samples were often collected from a series of depths at each site. There is a correlation between sample depth and oxygen deficit, with higher deficits occurring near the base of the channel. Other than samples collected at the surface, there were not a sufficient number of samples to perform a trend analysis. Consequently, only those samples collected at the surface were used to evaluate trends in the data.

The saturation concentration for dissolved oxygen was calculated for each of the samples using the Fair-Geyer expression. This expression states that:

$$C_s = \frac{5(100 - Cl)}{(T + 35)}$$

where C_s is DO saturation in mg/L, Cl is chlorinity in parts per thousand and T is temperature in degrees Celsius. Chlorinity was taken to be zero for this analysis.

The value of the sample results was subtracted from the saturation value to determine the oxygen deficit at the time the sample was taken. A negative value for the oxygen deficit indicates that the measured value was greater than the saturation value. There are several possible physical reasons that the water could be supersaturated; consequently, these values were retained in the data set. The data was then analyzed using oxygen deficit as the parameter of interest. The site locations and summary statistics are shown in Table 1.

Trend Analysis

Analysis of all samples collected at the water surface at Falls Creek indicates a significant improvement with time in the dissolved oxygen conditions at this site (Figure 1). However, there is also a systematic trend in the time of day the samples were collected. Samples collected near the beginning of the sampling program were taken earlier in the day (Figure 2) and there is a significant correlation between time of sample collection and the oxygen deficit. The deficits are significantly higher earlier in the day when photosynthesis has not replenished dissolved oxygen in the river (Figure 3).

Table 1 Summary Statistics for Dissolved Oxygen

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84 – 10/97	2/84 - 10/97	4/84 – 10/97	2/84 – 5/90
# of Samples	114	73	113	72
Average DO, mg/L	9.19	9.42	8.62	8.3
Median DO, mg/L	9.00	9.30	8.59	8
DO Std Deviation	1.77	1.94	1.47	1.7
Avg O ₂ Deficit, mg/L	0.11	-0.20	0.46	0.74
Median O ₂ Def., mg/L	0.24	-0.04	0.48	0.7
Deficit Std Dev	1.09	1.38	0.79	1.14

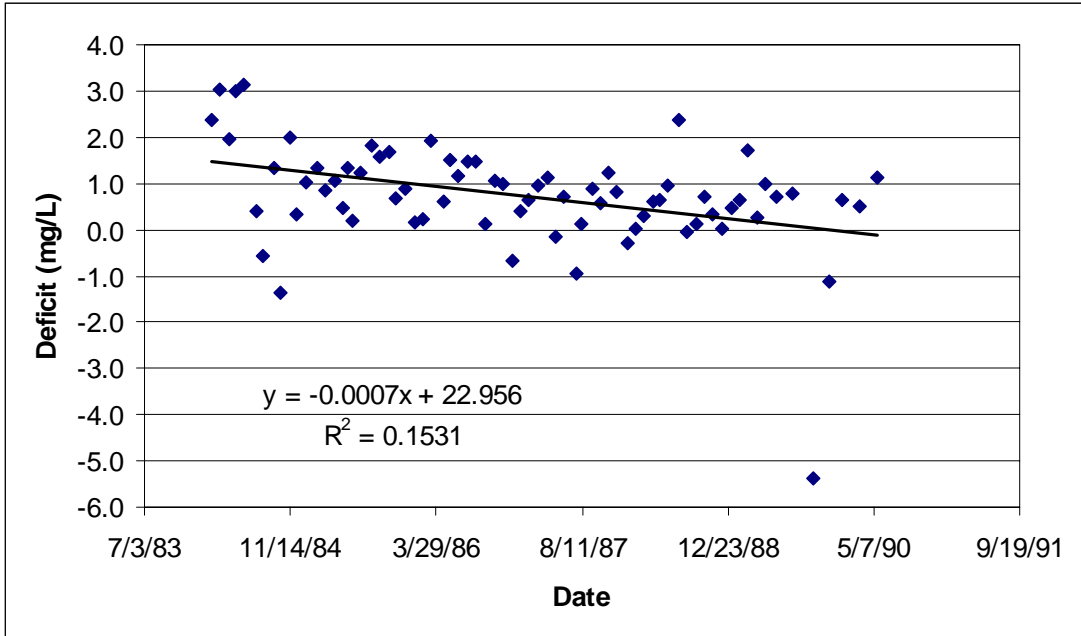


Figure 1 Oxygen Deficit at Falls Creek

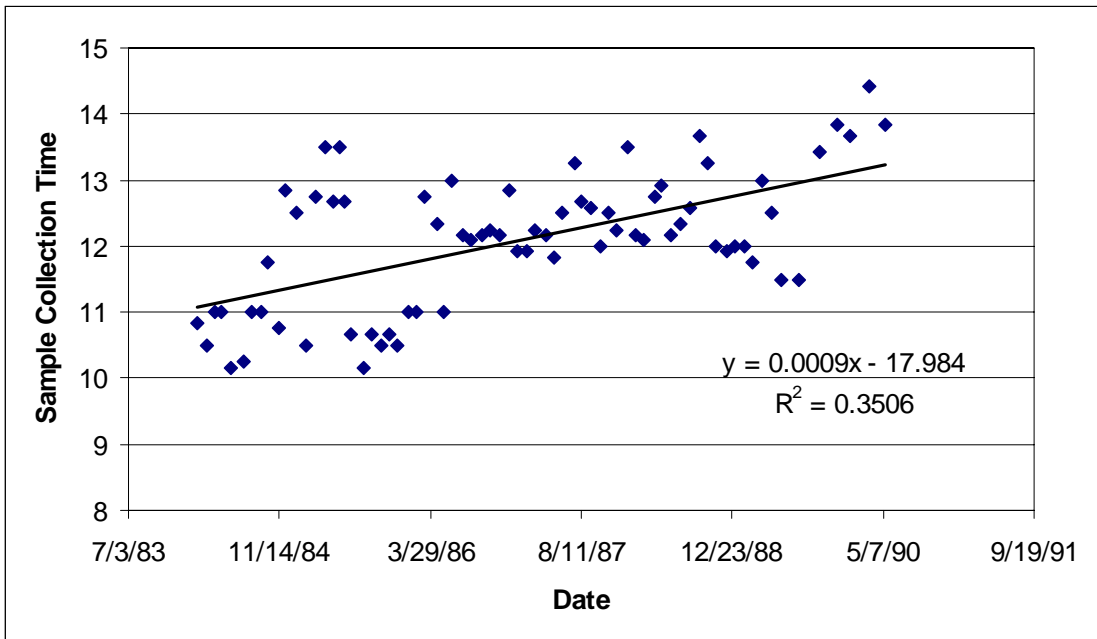


Figure 2 Sample Collection Times at Falls Creek

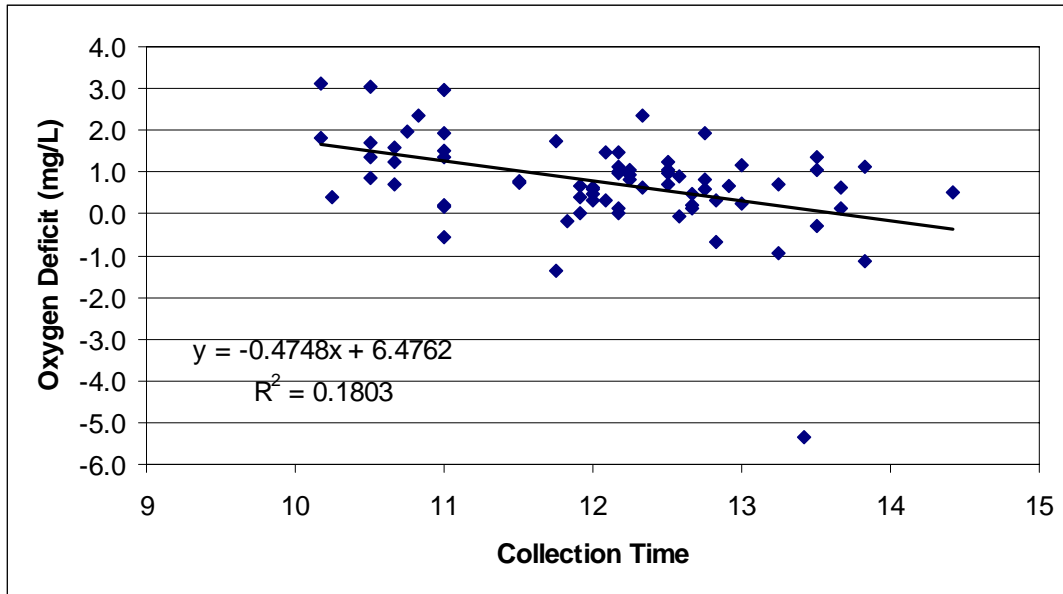


Figure 3 Relation between Collection Time and O₂ Deficit

The subtle shift in sampling protocol at this site creates such a strong signal in the data that it becomes extremely difficult to detect a correlation with land use changes or other events in the watershed. A multiple regression analysis was performed using oxygen deficit of samples collected at the surface as the dependent variable and time of day and date as the independent variables. The results are summarized in Table 11 and they indicate no significant change in oxygen deficit at the 90% confidence level.

A similar analysis was performed on the data collected at RR 1320. The sample collection at this site also changed systematically, with more recent samples collected earlier in the day. This made it appear that there was strong trend of reduced oxygen concentrations. A multiple linear regression analysis was conducted using date and time of day as the independent variables. This analysis indicated a slight trend of reduced oxygen concentrations even when time of sampling was taken into account. However, this trend was not significant at the 95% confidence level. The multiple regression statistics are shown in Table 12. The effect of season was also investigated at RR1320; however, this proved to be a completely random variable. Similar analyses were performed on the data collected at Johnson City and Hammett’s Crossing. The results of

the regression analyses are shown in Table 13 and Table 14. There was no significant long-term trend in oxygen deficit at either of these sites.

An ANOVA test was performed on the pooled raw data from the Pedernales, which indicated a significant spatial trend in oxygen deficit at a greater than 99% confidence level. The small difference between the mean deficits at RR 1320 and Johnson City and the mean deficits at RR 962 and Falls Creek are not significantly different. The deficits at RR 962 and Falls Creek are significantly higher than at either of the upstream stations. Table 15 contains the ANOVA statistics, while a bar graph of the average deficits is shown in Figure 4.

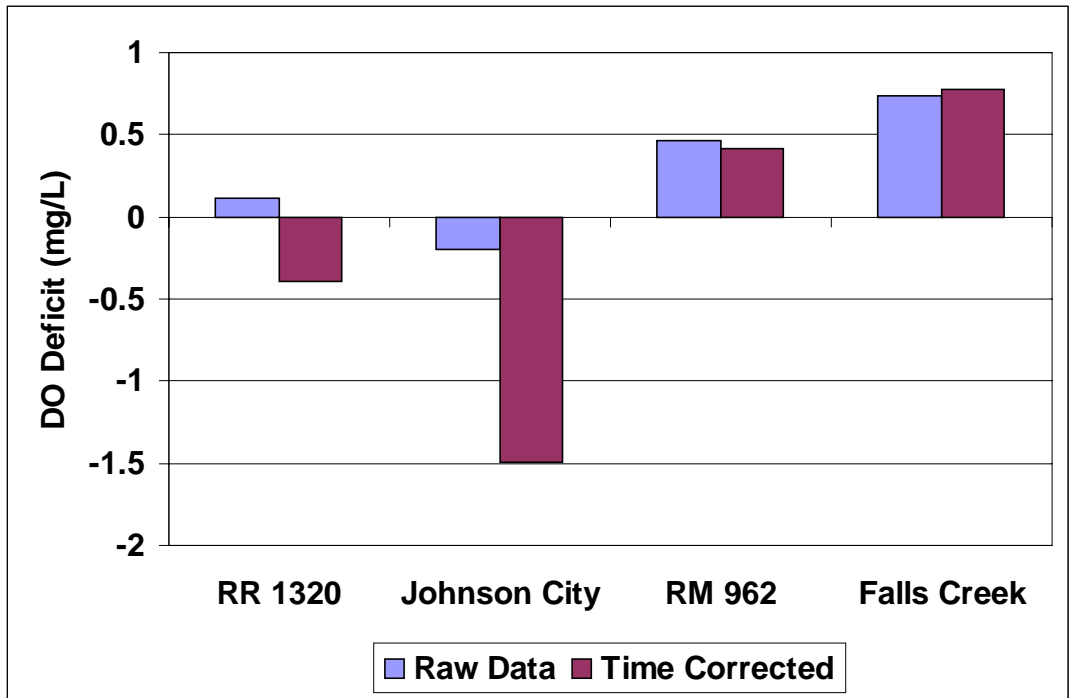


Figure 4 Average Oxygen Deficits on the Pedernales

The ANOVA analysis also was performed on the oxygen deficit data with a correction for sampling time. The data at each site was normalized to a common sampling time of noon. The test statistics for the normalized data shown in Table 16 indicate a more significant spatial trend than the raw data. The average value at each site based on a common sampling time is also shown in Figure 4.

Conclusions

Dissolved oxygen data from the Pedernales River collected between 1984 and 1997 indicate no significant long-term trends; however, the identification of changes in the dissolved oxygen concentrations has been hampered by systematic changes in the way the data have been collected. It is interesting to note, however, that 7 of the 8 highest oxygen deficits recorded on the Pedernales were measured during 1984. The data do indicate a trend of increasing oxygen deficit between Johnson City and the river segment boundary at Falls Creek. The dissolved oxygen conditions in the river are generally very good, even at the sites with higher deficits.

The large dissolved oxygen surplus measured at Johnson City could indicate a potential problem area. Large surpluses can be associated with algal blooms, which produce oxygen during the day via photosynthesis. Conversely, algal blooms can result in extremely low oxygen concentrations at night. Consequently, it is important to identify the reason for the high daytime readings at this site.

Specific Conductivity

Summary Statistics

The summary statistics for all the specific conductance data are shown in Table 2.

Table 2 Summary Statistics for Specific Conductivity

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84 – 10/97	2/84 - 10/97	4/84 – 10/97	2/84 – 5/90
# of Samples	114	73	113	72
Average Sp. Cond	682	648	540	553
Median Sp. Cond	690	623	546	546
Std Deviation	128	153	98	109

Trend Analysis

A temporal trend analysis was performed on the data set for each of the four monitoring sites. At two of the sites, the conductance was highly correlated with river discharge as measured at Johnson City. Consequently, a multiple linear regression was performed using date and flow as the independent variables. The regression statistics for each of the sites are shown in Table 17 through Table 20.

The regression equations indicate no significant temporal trend at Falls Creek and RM 962; however, Johnson City and RR 1320 sites both showed improvements in water quality during the monitoring period. This trend is driven mainly by very high conductance readings during the drought of 1984.

The data from Johnson City and RR 1320 were then analyzed using the Mann-Kendall test. This test confirms the downward trend in conductance at RR 1320, with a Z statistic of 2.39 indicating a probability of less than 0.008 that the trend could have occurred by chance. The analysis indicated no significant statistical relationship between date and conductance at Johnson City, where the Z statistic was 0.76 indicating a probability of 0.22 that the differences are the result of random fluctuations. A plot of the data collected at RR 1320 is shown in Figure 5.

An ANOVA test for spatial variation in conductance was conducted using the data from the four monitoring sites. The results of the test are shown in Table 21, which indicates that there is a significant spatial trend. The mean concentrations for the four sites are shown graphically in Figure 6 where the trend towards decreasing concentrations downstream is apparent.

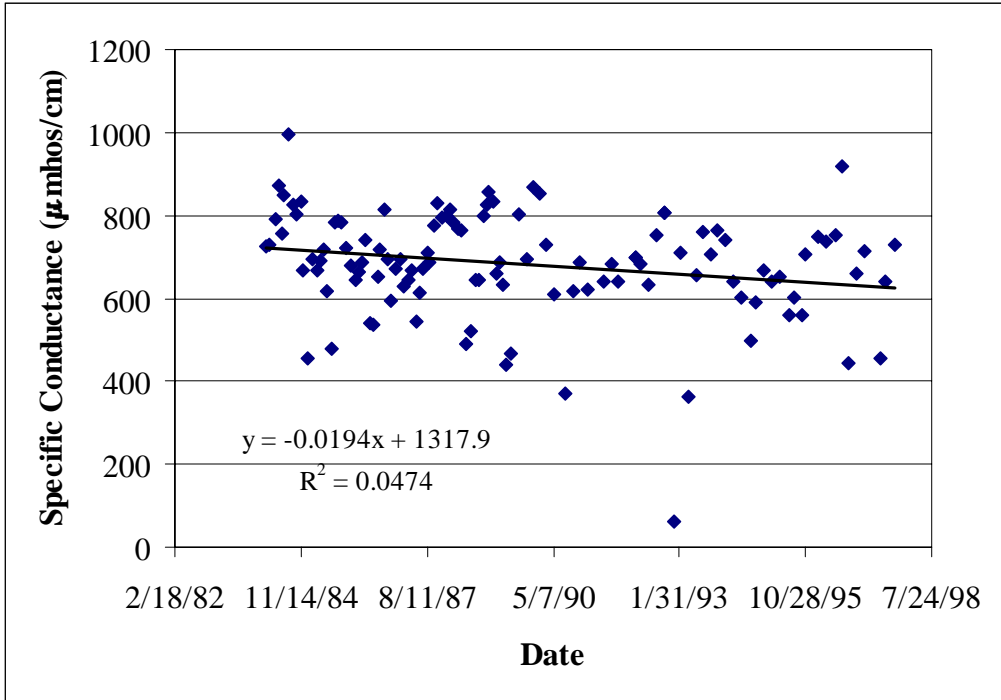


Figure 5 Specific Conductance at RR 1320

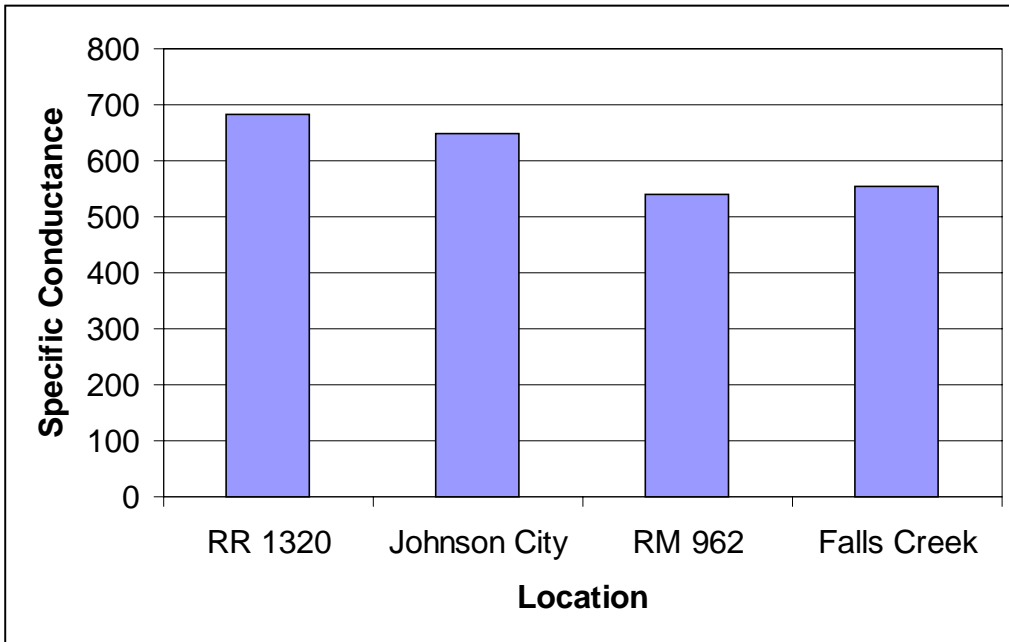


Figure 6 Comparison of Mean Specific Conductance

Conclusions

Conductance is clearly related to river discharge with higher readings occurring during low flow periods. This is a common relationship, since low flows are mainly created by groundwater inflows, which are generally higher in dissolved solids than storm runoff.

Only one site, RR1320, exhibited a statistically significant trend in specific conductance; however, there is a strong spatial trend in dissolved solids. The readings are highest at the upstream site and gradually decline to near the RM 962 site. This is likely the result of dilution by the dissolved solids concentrations by inflows between RR 1320 and RM 962. The high dissolved solids concentrations upstream may be the result of natural factors, but could also be due in part to human activities. High dissolved solids concentrations are often encountered in agricultural areas and can be caused by irrigation return flows. Further investigation is warranted to identify the causes, since total dissolved solids are listed as a constituent of possible concern in the LCRA Water Quality Assessment.

Chloride

Summary Statistics

Chloride is listed as a constituent of possible concern in the 1994 LCRA water quality assessment, which indicates that a concentration of 105 mg/L is considered a problem level. The summary data shown in Table 3 shows that the average concentrations at all four sites is well below this value.

Table 3 Summary Statistics for Chloride

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84-12/96	2/84-8/90	10/90-12/96	2/84 – 5/90
# of Samples	96	72	23	73
Average (mg/L)	66.2	62.5	37.3	40.7
Median (mg/L)	64.5	54	37	39
Std Deviation	22.8	32.3	11.2	20.5

Trend Analysis

Multiple linear regression was used initially to identify possible trends in the chloride data. The independent variables were selected as date and flow rate to account for higher salinity, which commonly is associated with low flow conditions. The regression statistics, which are shown in Table 22 through Table 25, indicate a trend of decreasing concentrations at RR 1320, Johnson City, and RM 962. The data at these sites was then subjected to the Mann-Kendall test. This test confirmed the significance of the trend at RR 1320 and RM 962, with Z statistics of 1.94 and 2.19 respectively, which corresponds to confidence levels of 0.027 and 0.014. For Johnson City, the calculated Z statistic was 0.57, which corresponds to a *P* value of 0.28. The data for RR 1320 and RM 962 are plotted in Figure 7 and Figure 8.

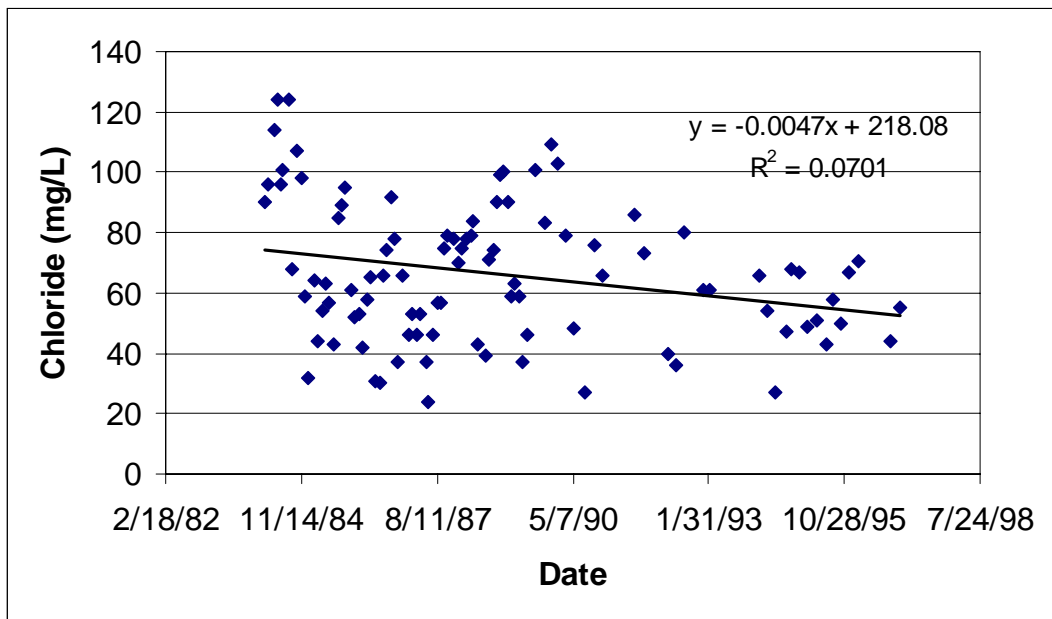


Figure 7 Chloride Concentrations at RR 1320

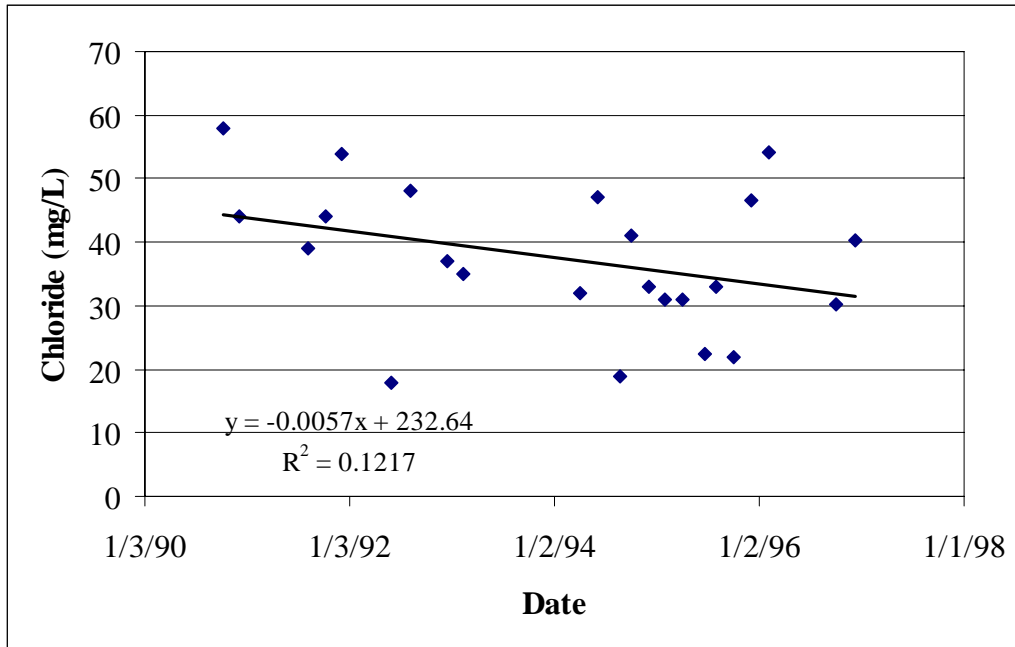


Figure 8 Chloride Concentrations at RM 962

An ANOVA test was performed on the pooled data from all four sites to determine whether spatial trends were present. The test statistics, which are shown in Table 26, indicate that a significant decrease in concentration occurs between Johnson City and RM 962. This decrease is evident in the graph shown in Figure 9.

Since chloride concentrations at many of the sites are significantly correlated with flow rate, the concentrations were normalized to the Pedernales median flow rate, 50 cfs. The ANOVA analysis was then performed on the normalized data. The normalized data indicate an even more statistically significant spatial trend. The averages for the normalized data set are also shown in Figure 9.

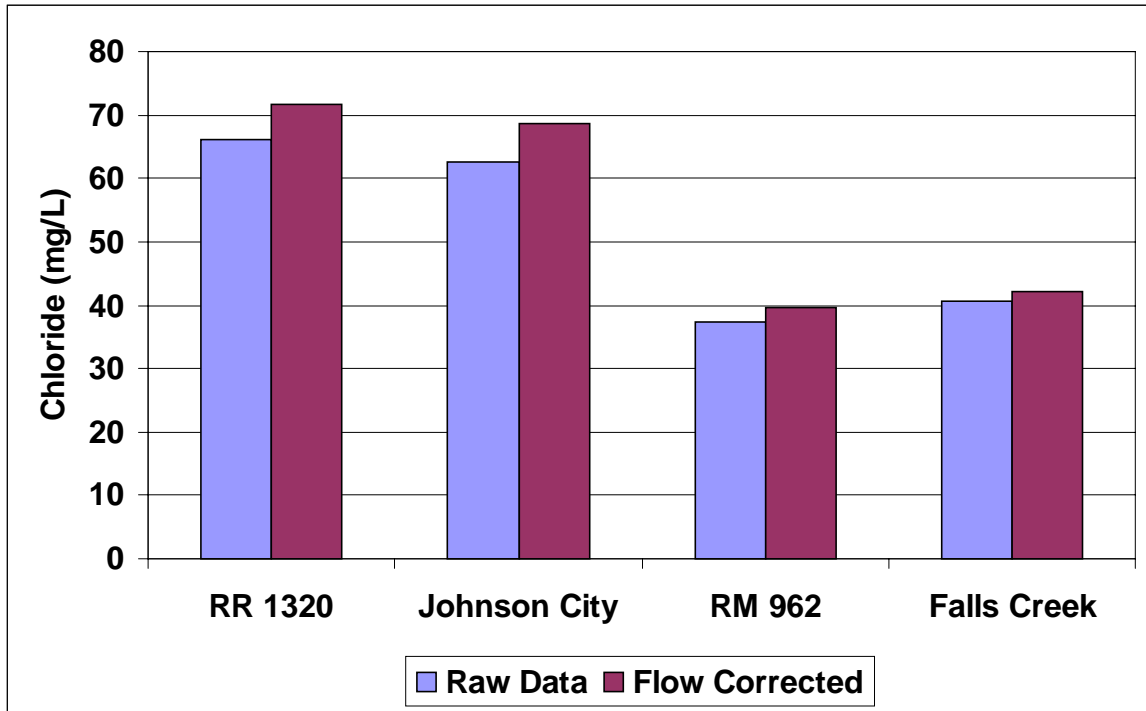


Figure 9 Average Chloride Concentrations

Sulfate

Summary Statistics

Sulfate is another constituent that the LCRA has identified of being of possible concerns because of high concentrations. The critical concentration for this constituent is 50 mg/L. The summary statistics for the four sites are shown in Table 4. The average values are much lower than the critical concentration.

Trend Analysis

Multiple linear regression, with date and flow as the independent variables, was used to identify temporal trends at each site. The regression statistics are shown in Table 28 through Table 31. According to the regression analysis, the only site with a significant temporal trend in RR 1320. The data from this site was also tested for significance using

the Mann-Kendall test. The Z statistic was 2.59, which corresponds to a *P* value of 0.005, confirming the temporal trend. The data from RR 1320 is shown in Figure 10.

Table 4 Summary Statistics for Sulfate

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84-12/96	2/84-8/90	10/90-12/96	2/84 – 5/90
# of Samples	95	72	23	73
Average (mg/L)	32.2	36.6	26.0	27.4
Median (mg/L)	33	33	26	27
Std Deviation	8.6	28.0	6.4	11.2

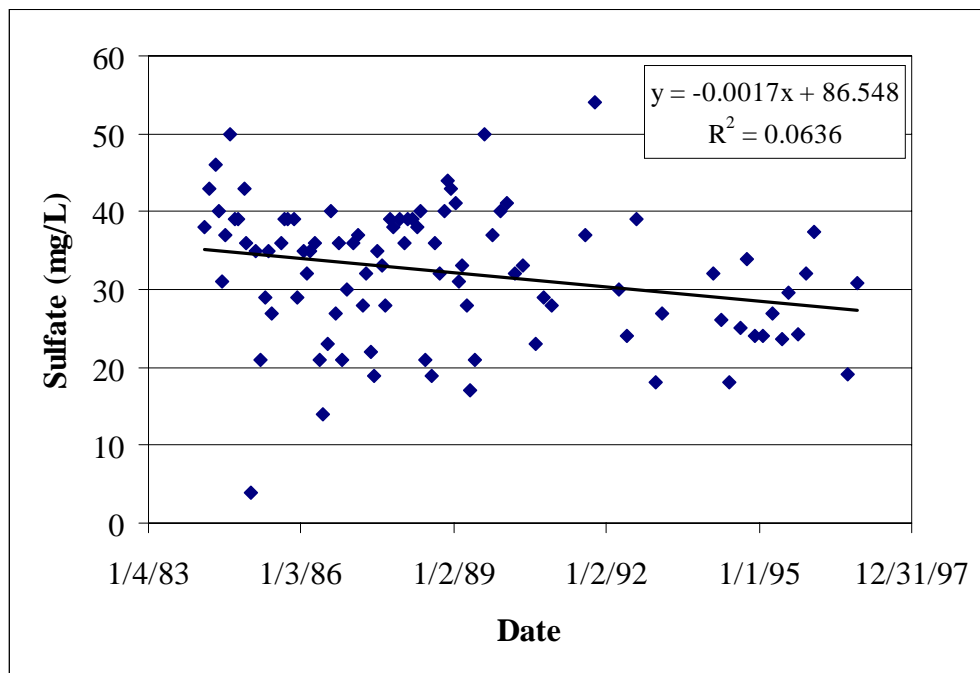


Figure 10 Sulfate Concentrations at RR 1320

The ANOVA analysis was then used to determine whether there were significant differences among the sites. The ANOVA statistics are shown in Table 32 and they indicate that the concentrations at Johnson City are significantly higher than at the two downstream monitoring stations. The concentrations at RR 1320 are also higher than the

downstream sites; however, the difference is not significant. The average concentrations at the sites are shown in Figure 11.

The ANOVA analysis was also performed with data normalized to the median flow rate of 50 cfs. This data set also had a significant spatial variation, which had a higher degree of confidence than the raw data. The average values for the normalized data are also presented in Figure 10.

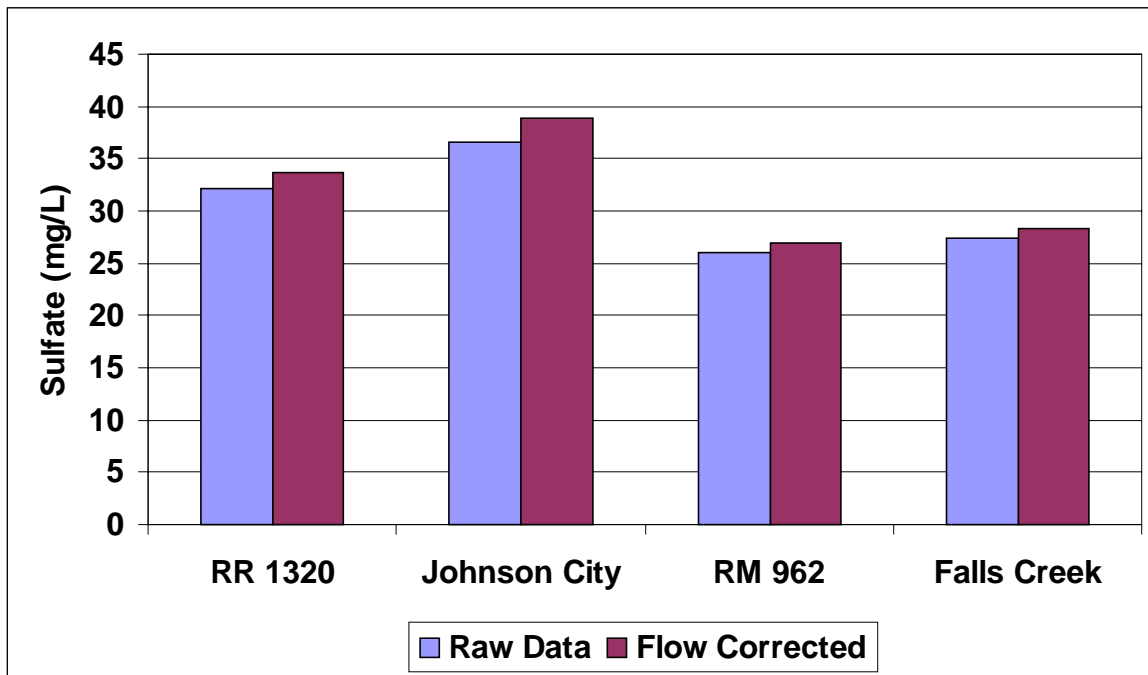


Figure 11 Average Sulfate Concentrations

Nitrate plus Nitrite

Summary Statistics

Nutrients are listed as a constituent of concern in the LCRA 1994 Water Quality Assessment and the nitrogen concentrations are relatively high compared to many surface water bodies. The average concentrations and other summary statistics for the four monitoring sites are shown in Table 5. The average values are much higher than the

median values compared to other constituents indicating that the underlying distribution is skewed.

Table 5 Summary Statistics for Nitrate plus Nitrite

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84-10/95	2/84-8/90	10/90-12/96	2/84 – 5/90
# of Samples	92	72	13	73
Average (mg/L)	0.36	0.28	0.21	0.19
Median (mg/L)	0.18	0.095	0.16	0.08
Std Deviation	0.43	0.32	0.23	0.21

Trend Analysis

Multiple linear regression, using flow and date as the independent variables, was used to test for a temporal trend at each of the monitoring sites. The regression statistics for each of the sites are shown in Table 34 through Table 37. The only site with a statistically significant trend was RR 1320, which showed a decrease in concentration through time. The data at this site was then analyzed using the Mann-Kendall test. The calculated Z statistic was 1.19, which corresponds to a *P* value of 0.117 and indicates that the trend is not statistically significant.

An ANOVA analysis was performed on the data to determine whether there were differences among the sites and the results are shown in Table 38. The analysis indicated a significant difference at the 0.009 confidence level. A plot of the average concentration at each site is shown in Figure 12 and the trend to decreasing concentrations downstream is readily apparent.

This reduction in concentration could be the result of uptake of the nutrients in the river or dilution by inflows with lower nitrogen concentrations. The fact that specific conductance shows this same trend suggests that the reduction is due primarily to

dilution. The higher concentrations upstream could be caused by naturally high levels of nitrogen in groundwater contributing baseflow to the river, use of fertilizers in agricultural areas, or discharge from wastewater treatment plants.

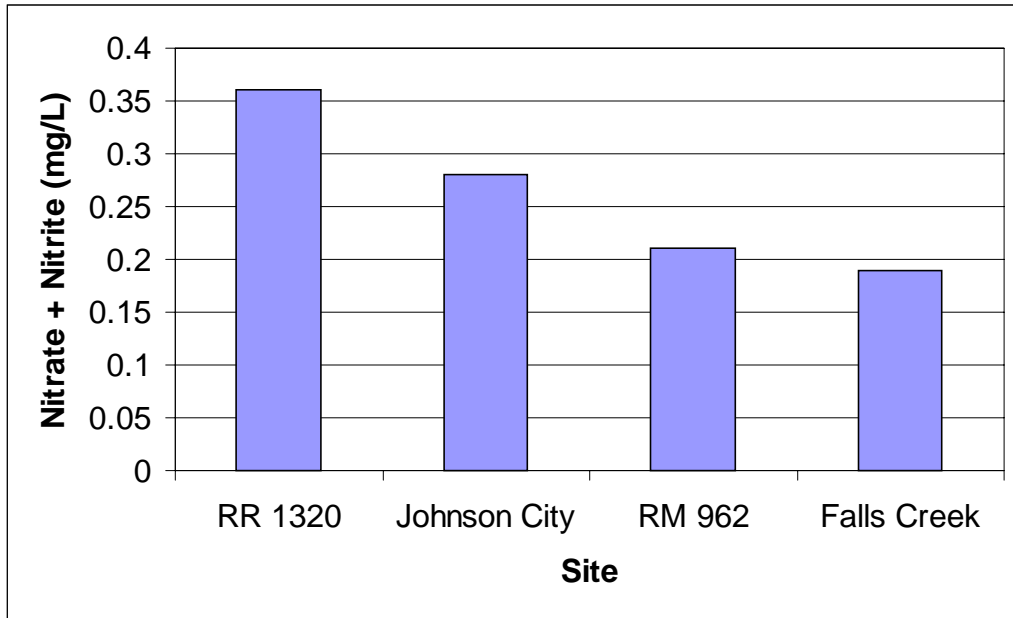


Figure 12 Average Nitrate plus Nitrite Concentrations

Total Kjeldahl Nitrogen

Summary Statistics

Another important form of nitrogen is total Kjeldahl nitrogen (TKN), which includes ammonia plus organic nitrogen. The summary statistics for each of the sites is shown in Table 6.

Trend Analysis

Multiple linear regression, using flow and date as the independent variables, was used to test for a temporal trend at each of the monitoring sites. The regression statistics for each

of the sites are shown in Table 39 through Table 42. The regression analysis indicates that the only significant trend occurs at Johnson City, where the concentrations are decreasing with time (Figure 13). The data from Johnson City were then examined with the Mann-Kendall test. The Z statistic for this test was 1.61, which corresponds to a P value of 0.054, and indicates that the slope is not statistically significant at the 95% confidence level.

Table 6 Summary Statistics for Total Kjeldahl Nitrogen

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84-12/96	2/84-8/90	10/90-12/96	2/84 – 5/90
# of Samples	96	72	23	73
Average (mg/L)	0.58	0.69	0.53	0.73
Median (mg/L)	0.49	0.535	0.277	0.54
Std Deviation	0.44	0.50	0.75	0.79

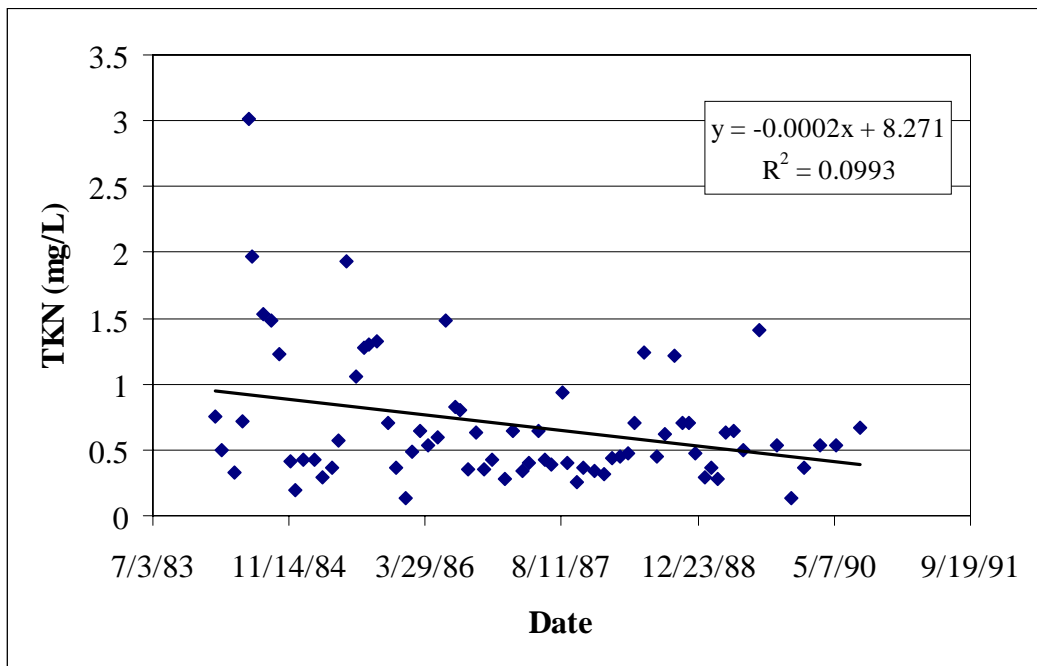


Figure 13 Total Kjeldahl Nitrogen at Johnson City

An ANOVA test was conducted on the TKN data and the results are shown in Table 43. Although there is a general trend towards decreasing concentrations in the downstream direction, the differences are not statistically significant ($P = 0.354$).

Ammonia

Summary Statistics

Ammonia can be toxic to aquatic life at low concentrations and therefore is a critical water quality parameter. The high dissolved oxygen concentrations in the Pedernales are oxidize the ammonia to nitrate, reducing the concentration of ammonia. The average concentrations and other summary statistics are shown in Table 7.

Table 7 Summary Statistics for Ammonia

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84-12/96	2/84-8/90	10/90-12/96	2/84 – 5/90
# of Samples	94	72	23	73
Average (mg/L)	0.054	0.070	0.058	0.069
Median (mg/L)	0.02	0.035	0.04	0.03
Std Deviation	0.10	0.117	0.062	0.097

Trend Analysis

Multiple linear regression, with flow and date as the independent variables, was used to detect the presence of temporal trends. The regression statistics shown in Table 44 through Table 47 indicated no significant trend at any of the sites.

An ANOVA analysis was used to detect differences in ammonia concentrations among the sites. The ANOVA statistics, which are shown in Table 48, indicate no significant differences in concentration.

Total Phosphorus

Summary Statistics

The summary statistics for phosphorus are shown in Table 8 and they indicated low median concentrations at all monitoring sites.

Table 8 Summary Statistics for Total Phosphorus

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84-12/96	2/84-8/90	10/90-12/96	2/84 – 5/90
# of Samples	96	72	23	73
Average (mg/L)	0.046	0.148	0.071	0.033
Median (mg/L)	0.02	0.05	0.03	0.02
Std Deviation	0.08	0.61	0.141	0.035

Trend Analysis

Multiple linear regression, using flow and date as the variables indicated a significant trend only at Johnson City. The regression statistics for this analysis are shown in Table 49 through Table 52. The Mann-Kendall test was performed on the data from Johnson City to confirm the trend. The value of the Z statistic was 0.81, which corresponds to a P value of 0.209, indicating no significant trend.

Total Organic Carbon

Summary Statistics

The summary statistics shown in Table 9 demonstrate the relatively low concentrations of Total Organic Carbon (TOC) present in the Pedernales River.

Table 9 Summary Statistics for Total Organic Carbon

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84-12/96	2/84-8/90	10/90-12/96	2/84 – 5/90
# of Samples	96	72	23	73
Average (mg/L)	3.69	4.02	3.12	3.29
Median (mg/L)	3	3	3	3
Std Deviation	1.81	2.40	1.34	1.51

Trend Analysis

Multiple linear regression, using date and flow as the independent variables, indicates that the only statistically significant temporal occurs at Johnson City. The statistics for the linear regression test are shown in Table 54 through Table 57. The data were analyzed with the Mann-Kendall test to confirm the trend. The Z statistic for this test was 1.01, which corresponds to a P value of 0.156, indicating that the slope is not significant.

The ANOVA analysis indicated no significant differences in concentration among the stations. The ANOVA statistics are shown in Table 58.

Fecal Coliform

Summary Statistics

The summary statistics for the fecal coliform data at the four monitoring sites are shown in Table 10. The large difference between the average and median values for all the sites indicates that the underlying distribution is not normal. Consequently, the median is a better representation of the normal conditions in the river than the average value. All of the median values are well below standards for contact recreation.

Table 10 Summary Statistics for Fecal Coliform Data

Parameter	RR 1320	Johnson City	RM 962	Falls Creek
Period of Record	2/84-12/96	2/84-8/90	10/90-12/96	2/84 – 5/90
# of Samples	96	72	23	73
Average Count/100 ml	280	332	311	328
Median Count/100 ml	43	75	36	16
Std Deviation	680	1190	803	1698

Trend Analysis

Multiple linear regression was used to identify significant temporal trends in the water quality data. The independent variables were time and flow rate. Flow rate was selected to try to distinguish between conditions of baseflow and storm runoff; however, flow was not statistically significant at any of the sites. The regression analysis indicated that there was no temporal trend at any of the sites. The regression statistics for fecal coliform at the four sites are shown in Table 59 through Table 62.

ANOVA test for difference in Fecal Coliform means for all sites on the Pedernales indicate no statistically significant spatial trend. The ANOVA statistics are shown in Table 63.

Conclusions and Recommendations

The LCRA water quality sampling program for the Pedernales River has focused primarily on the quality of ambient, baseflow conditions. Of the 264 samples collected and analyzed for Fecal coliform and other common constituents, only 18 (7%) were collected during peak flow conditions caused by stormwater runoff. The current sampling program is best suited for identifying long term changes in water quality caused by continuous point source discharges and by significant changes in groundwater quality.

Reduction in groundwater quality resulting from land use changes would have to be widespread or very severe to cause statistically significant changes in the quality of the Pedernales and other rivers in the LCRA service area. In general, agricultural land uses have a greater impact on groundwater quality than urban land uses. Consequently, one would expect the most commonly identified impacts to be associated with agricultural activities and be reflected in higher concentrations of dissolved solids and nutrients. Urbanization of a watershed generally affects the water quality of the river during ambient conditions by increased discharges from wastewater treatment facilities. These discharges are often associated with a reduction in dissolved oxygen concentrations. Urbanization also can result in reduction of baseflow because the increased impervious cover prevents rainfall from infiltrating; however, the urban area would have to cover a large portion of the watershed for the changes to be obvious.

Analysis of the LCRA water quality database for the Pedernales River indicates that several constituents exhibit a significant spatial trend. All of the constituents that varied had higher concentrations upstream than down. These constituents are dissolved oxygen, chloride, sulfate, nitrate plus nitrite, and specific conductance. The monitoring data are not sufficient for identifying the reasons for these changes, which may be natural, manmade, or a combination of the two.

The higher levels of dissolved solids, nitrogen, and other ionic species may be derived from groundwater inflow to the river. Groundwater often contains higher concentrations

of these constituents than surface water. Elevated concentrations of these constituents also are associated with agricultural activities. Irrigation return flows have higher levels of dissolved solids and can carry nutrients derived from fertilizers as well as agricultural pesticides. Measurements of pesticide concentrations were not included in the monitoring program. Determination of the causes of the higher concentrations should be the focus of future studies, because dissolved solids, chloride, and sulfate are all listed as parameters of possible concern in the 1994 LCRA water quality assessment (LCRA, 1994). Concentrations of these constituents are currently below levels that caused them to be flagged in the 1994 report.

Temporal trends were most evident at the upstream monitoring station at RR 1320, which generally had the longest period of record. Constituents with significant temporal trends include specific conductance, chloride, and sulfate. The concentrations of each of these showed a reduction with time. The monitoring site at RM 962 showed a single parameter with a temporal trend, chloride, which also had declining concentrations. There was no significant temporal trend for any constituent at the Johnson City and Falls Creek sites. These reductions in concentration may be the result of much higher than average rainfall in 1991 and 1992. Rainfall generally has very low concentrations of these constituents, causing a dilution in groundwater, which contributes baseflow to the river, as well as in the river itself through increased surface runoff.

The water quality of the Pedernales River can be characterized as very good and supportive of all designated beneficial uses. Current concentrations of all monitored water quality constituents are generally below levels that would be cause for concern or result in the listing of this river segment as impaired. In addition, this data set indicates that the concentrations of the analyzed constituents at each monitoring site are relatively constant or improving. There is no immediate concern that changes in land use or other human activities threaten the water quality of the Pedernales River; however, the source of higher concentrations of dissolved solids and nutrients in the upstream reach of the river should be identified. The change from undeveloped to agricultural land use

occurred many years before the beginning of any water quality monitoring programs and may be responsible for higher constituent concentrations in this portion of the watershed.

A consistent sampling protocol is required to reduce the noise inherent in the dissolved oxygen and other environmental data. For dissolved oxygen, important variables in the sampling program are collection time and sampling depth. In order to avoid a situation where sampling time changes systematically during the monitoring period or where sampling times differ systematically between monitoring stations, collection times should be randomized.

Despite the generally high quality of the Pedernales River water, eight fish kill episodes have been reported in the river, including five since 1990. Approximately half of the kills have been the result of illegal dumping of toxic substances. These episodes highlight the importance of an effective public education/outreach program. Such a program can make citizens more aware of the environmental impacts of improper disposal of waste materials. It is especially important to target owners of small businesses, which often need to dispose of significant quantities of spent solvents, lubricants, paint and other toxic materials.

The remaining fish kills were the result of wastewater treatment plant (WWTP) discharges to the Pedernales near Johnson City, resulting in low dissolved oxygen concentrations. Conventional water quality monitoring programs are not effective for identifying episodic events resulting from equipment malfunction or other causes and samples collected within a few weeks of the events did not have particularly low dissolved oxygen concentrations. Wastewater discharges may be a factor in the trend of decreasing dissolved oxygen concentrations from upstream to downstream. The number of fish kills related to wastewater discharges suggests that a review of the permit requirements and adequacy of the Johnson City WWTP should be a high priority.

Bibliography

Gilbert, R.O., 1987, *Statistical Methods for Environmental Pollution Monitoring*, Van Nostrand Reinhold Company, New York.

LCRA, 1994, *1994 Water Quality Assessment of the Colorado River Basin*.

Minitab Inc, 1998, *Minitab for Windows: Release 12*, State College, PA.

Appendix A

Table 11 Regression Statistics for Oxygen Deficit at Falls Creek

Predictor	Coef	StDev	T	P
Constant	16.981	6.635	2.56	0.013
DATE	-0.0003847	0.0002371	-1.62	0.109
Time	-0.3322	0.1484	-2.24	0.028

S = 1.027 R-Sq = 21.0% R-Sq(adj) = 18.8%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	19.415	9.708	9.20	0.000
Residual Error	69	72.836	1.056		
Total	71	92.252			

Source	DF	Seq SS
DATE	1	14.125
Time	1	5.290

Table 12 Regression Statistics for Oxygen Deficit at RR 1320

Predictor	Coef	StDev	T	P
Constant	0.018	2.725	0.01	0.995
DATE	0.00012079	0.00006925	1.74	0.084
time	-0.34966	0.08567	-4.08	0.000
Season	-0.04620	0.08257	-0.56	0.577

S = 0.9904 R-Sq = 20.3% R-Sq(adj) = 18.1%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	3	27.1926	9.0642	9.24	0.000
Residual Error	109	106.9077	0.9808		
Total	112	134.1003			

Source	DF	Seq SS
DATE	1	10.5199
Time	1	16.3656
Season	1	0.3071

Table 13 Regression Statistics for Oxygen Deficit at Johnson City

Predictor	Coef	StDev	T	P
Constant	7.917	6.192	1.28	0.205
DATE	-0.0000683	0.0001923	-0.36	0.724
Time_(d)	-0.6022	0.1711	-3.52	0.001

S = 1.289 R-Sq = 15.6% R-Sq(adj) = 13.2%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	21.445	10.722	6.45	0.003
Residual Error	70	116.351	1.662		
Total	72	137.796			

Source	DF	Seq SS
DATE	1	0.850
Time_(d)	1	20.595

Table 14 Regression Statistics for Oxygen Deficit at Hammett's Crossing

Predictor	Coef	StDev	T	P
Constant	0.307	2.384	0.13	0.898
DATE	0.00004533	0.00006302	0.72	0.474
Time_d	-0.11427	0.03820	-2.99	0.003

S = 0.7344 R-Sq = 15.7% R-Sq(adj) = 14.2%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	11.0656	5.5328	10.26	0.000
Residual Error	110	59.3270	0.5393		
Total	112	70.3927			

Source	DF	Seq SS
DATE	1	6.2402
Time_d	1	4.8254

Table 15 ANOVA Statistics for Oxygen Deficit

Source	DF	SS	MS	F	P
Factor	3	43.96	14.65	10.07	0.000
Error	372	541.44	1.46		
Total	375	585.40			

Level	N	Mean	StDev	Individual 95% CIs For Mean Based on Pooled StDev	
-				-----+-----+-----+-----	
Falls Cr	73	0.741	1.140		(-----*-----)
RM 962	114	0.546	1.237		(-----*-----)
Johnson	74	-0.202	1.383	(-----*-----)	
RR 1320	115	0.108	1.089	(-----*-----)	
-				-----+-----+-----+-----	
Pooled StDev =		1.206		0.00	0.50 1.00

Table 16 ANOVA Statistics for DO Corrected for Time

Analysis of Variance					
Source	DF	SS	MS	F	P
Factor	3	236.463	78.821	79.92	0.000
Error	367	361.956	0.986		
Total	370	598.418			

Level	N	Mean	StDev	Individual 95% CIs For Mean Based on Pooled StDev	
-				-+-----+-----+-----+-----	
1320noon	113	-0.3900	0.9918		(--*--)
johnoon	73	-1.4900	1.2724	(--*--)	
962noon	113	0.4200	0.7295		(--*--)
fcnoon	72	0.7800	1.0320		(--*--)
-				-+-----+-----+-----+-----	
Pooled StDev =		0.9931		-1.60	-0.80 -0.00 0.80

Table 17 Regression Statistics for Specific Conductance at Falls Creek

Predictor	Coef	StDev	T	P
Constant	383.0	642.3	0.60	0.553
DATE	0.00556	0.02021	0.28	0.784
Flow (cf	-0.03527	0.02589	-1.36	0.178

S = 108.6 R-Sq = 2.7% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	22461	11231	0.95	0.391
Residual Error	69	813739	11793		
Total	71	836200			

Source	DF	Seq SS
DATE	1	583
Flow (cf	1	21878

Table 18 Regression Statistics for Specific Conductance at Johnson City

The regression equation is

$$\text{CONDUCTANCE (UMHOS/CM @ 25C)} = 2102 - 0.0446 \text{ DATE} - 0.240 \text{ Flow (cfs)}$$

Predictor	Coef	StDev	T	P
Constant	2101.7	680.9	3.09	0.003
DATE	-0.04465	0.02135	-2.09	0.040
Flow (cf	-0.24019	0.08798	-2.73	0.008

S = 143.8 R-Sq = 14.1% R-Sq(adj) = 11.6%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	237438	118719	5.74	0.005
Residual Error	70	1446592	20666		
Total	72	1684030			

Source	DF	Seq SS
DATE	1	83408
Flow (cf	1	154031

Table 19 Regression Statistics for Specific Conductance at Hammett's Crossing

Predictor	Coef	StDev	T	P
Constant	493.6	217.3	2.27	0.025
DATE	0.001824	0.006636	0.27	0.784
Flow (cf	-0.07132	0.03288	-2.17	0.032

S = 97.03 R-Sq = 4.1% R-Sq(adj) = 2.4%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	44381	22190	2.36	0.100
Residual Error	109	1026228	9415		
Total	111	1070608			

Source	DF	Seq SS
DATE	1	87
Flow (cf	1	44294

Table 20 Regression Statistics for Specific Conductance at RR 1320

Predictor	Coef	StDev	T	P
Constant	1361.1	268.1	5.08	0.000
DATE	-0.020420	0.008190	-2.49	0.014
Flow (cf	-0.04861	0.04700	-1.03	0.303

S = 123.4 R-Sq = 6.8% R-Sq(adj) = 5.1%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	121217	60608	3.98	0.021
Residual Error	110	1673716	15216		
Total	112	1794933			

Source	DF	Seq SS
DATE	1	104944
Flow (cf	1	16273

Table 21 ANOVA Statistics for Specific Conductance

Source	DF	SS	MS	F	P
Factor	3	1483597	494532	33.32	0.000
Error	368	5461561	14841		
Total	371	6945158			

Level	N	Mean	StDev
RR 1320	114	682.1	127.8
Johnson	73	647.8	152.9
RM 962	113	539.9	98.8
Falls C	72	553.0	108.5

Individual 95% CIs For Mean
Based on Pooled StDev

Level	N	Mean	StDev	Lower CI	Upper CI
RR 1320	114	682.1	127.8	520.0	844.2
Johnson	73	647.8	152.9	340.0	955.6
RM 962	113	539.9	98.8	340.0	739.8
Falls C	72	553.0	108.5	335.0	771.0

Pooled StDev = 121.8

Table 22 Regression Statistics for Chloride at RR 1320

The regression equation is
 CHLORIDE (MG/L AS CL) = 229 - 0.00473 DATE - 0.0659 FLOW

Predictor	Coef	StDev	T	P
Constant	228.55	46.73	4.89	0.000
DATE	-0.004730	0.001438	-3.29	0.001
FLOW	-0.065859	0.009561	-6.89	0.000

S = 18.10 R-Sq = 38.4% R-Sq(adj) = 37.1%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	19001.9	9501.0	29.02	0.000
Residual Error	93	30451.7	327.4		
Total	95	49453.6			

Source	DF	Seq SS
DATE	1	3465.0
FLOW	1	15536.9

Table 23 Regression Statistics for Chloride at Johnson City

The regression equation is

CHLORIDE (MG/L AS CL) = 444 - 0.0117 DATE - 0.0765 FLOW

Predictor	Coef	StDev	T	P
Constant	444.3	162.1	2.74	0.008
DATE	-0.011685	0.005095	-2.29	0.025
FLOW	-0.07649	0.01722	-4.44	0.000

S = 28.10 R-Sq = 26.7% R-Sq(adj) = 24.5%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	19817.9	9909.0	12.55	0.000
Residual Error	69	54488.1	789.7		
Total	71	74306.0			

Source	DF	Seq SS
DATE	1	4241.9
FLOW	1	15576.0

Table 24 Regression Statistics for Chloride at RM 962

The regression equation is

CHLORIDE (MG/L AS CL) = 272 - 0.00670 DATE - 0.0372 FLOW

Predictor	Coef	StDev	T	P
Constant	271.87	96.90	2.81	0.011
DATE	-0.006700	0.002814	-2.38	0.027
FLOW	-0.03722	0.01189	-3.13	0.005

S = 9.021 R-Sq = 41.1% R-Sq(adj) = 35.2%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	1134.01	567.00	6.97	0.005
Residual Error	20	1627.52	81.38		
Total	22	2761.53			

Source	DF	Seq SS
DATE	1	336.18
FLOW	1	797.83

Table 25 Regression Statistics for Chloride at Falls Creek

The regression equation is
 CHLORIDE (MG/L AS CL) = - 35 + 0.00244 DATE - 0.0109 FLOW

Predictor	Coef	StDev	T	P
Constant	-34.9	117.4	-0.30	0.767
DATE	0.002444	0.003696	0.66	0.511
FLOW	-0.010858	0.004771	-2.28	0.026

S = 20.01 R-Sq = 7.3% R-Sq(adj) = 4.7%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	2207.6	1103.8	2.76	0.070
Residual Error	70	28033.0	400.5		
Total	72	30240.6			

Source	DF	Seq SS
DATE	1	133.1
FLOW	1	2074.5

Table 26 ANOVA Statistics for Chloride

Analysis of Variance for CHLORIDE

Source	DF	SS	MS	F	P
STATION	3	38190	12730	21.11	0.000
Error	260	156762	603		
Total	263	194952			

Individual 95% CIs For Mean
 Based on Pooled StDev

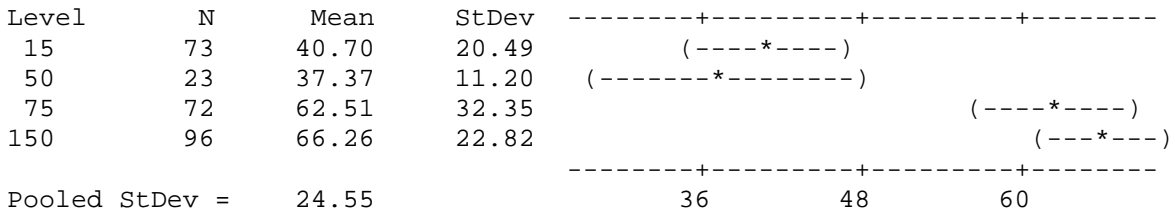


Table 27 ANOVA Statistics for Chloride Normalized for Flow

The regression equation is

$$\text{SULFATE (MG/L AS SO}_4\text{)} = -153 + 0.00607 \text{ DATE} - 0.0283 \text{ FLOW}$$

Predictor	Coef	StDev	T	P
Constant	-152.9	158.9	-0.96	0.339
DATE	0.006073	0.004993	1.22	0.228
FLOW	-0.02833	0.01688	-1.68	0.098

S = 27.54 R-Sq = 5.8% R-Sq(adj) = 3.1%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	3242.6	1621.3	2.14	0.126
Residual Error	69	52330.3	758.4		
Total	71	55572.9			

Source	DF	Seq SS
DATE	1	1105.1
FLOW	1	2137.5

Table 30 Regression Statistics for Sulfate at RM 962

The regression equation is

$$\text{SULFATE (MG/L AS SO}_4\text{)} = 101 - 0.00214 \text{ DATE} - 0.0155 \text{ FLOW}$$

Predictor	Coef	StDev	T	P
Constant	101.16	64.59	1.57	0.133
DATE	-0.002136	0.001876	-1.14	0.268
FLOW	-0.015491	0.007921	-1.96	0.065

S = 6.012 R-Sq = 18.9% R-Sq(adj) = 10.8%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	168.76	84.38	2.33	0.123
Residual Error	20	722.94	36.15		
Total	22	891.70			

Source	DF	Seq SS
DATE	1	30.52
FLOW	1	138.23

Table 31 Regression Statistics for Sulfate at Falls Creek

The regression equation is

$$\text{SULFATE (MG/L AS SO}_4\text{)} = 51.7 - 0.00072 \text{ DATE} - 0.00653 \text{ FLOW}$$

Predictor	Coef	StDev	T	P
Constant	51.66	63.75	0.81	0.420
DATE	-0.000725	0.002006	-0.36	0.719
FLOW	-0.006533	0.002590	-2.52	0.014

S = 10.86 R-Sq = 8.6% R-Sq(adj) = 6.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	775.4	387.7	3.29	0.043
Residual Error	70	8260.1	118.0		
Total	72	9035.5			

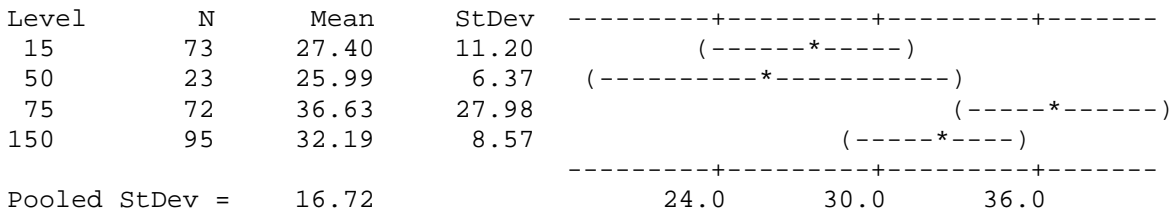
Source	DF	Seq SS
DATE	1	24.5
FLOW	1	750.8

Table 32 ANOVA Statistics for Sulfate

Analysis of Variance for SULFATE

Source	DF	SS	MS	F	P
STATION	3	3863	1288	4.61	0.004
Error	259	72411	280		
Total	262	76274			

Individual 95% CIs For Mean
Based on Pooled StDev



Pooled StDev = 16.72

* NOTE * N missing = 1

Table 33 ANOVA Statistics for Sulfate Normalized for Flow

Analysis of Variance

Source	DF	SS	MS	F	P
Factor	3	4621	1540	5.85	0.001

Error	259	68138	263	
Total	262	72759		

Individual 95% CIs For Mean
Based on Pooled StDev

Level	N	Mean	StDev	
150	95	33.70	7.75	(-----*-----)
75	72	38.89	27.44	(-----*-----)
50	23	27.60	5.92	(-----*-----)
15	73	28.60	10.72	(-----*-----)

Pooled StDev =	16.22	24.0	30.0	36.0	42.0
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Table 34 Regression Statistics for Nitrate plus Nitrite at RR 1320

The regression equation is
 NITRITE PLUS NITRATE, TOTAL 1 D = 2.79 -0.000077 DATE +0.000511 FLOW

91 cases used 5 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	2.787	1.236	2.25	0.027
DATE	-0.00007706	0.00003825	-2.01	0.047
FLOW	0.0005113	0.0002224	2.30	0.024

S = 0.4189 R-Sq = 9.3% R-Sq(adj) = 7.2%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	1.5787	0.7893	4.50	0.014
Residual Error	88	15.4414	0.1755		
Total	90	17.0201			

Source	DF	Seq SS
DATE	1	0.6514
FLOW	1	0.9273

Table 35 Regression Statistics for Nitrate plus Nitrite at Johnson City

The regression equation is

NITRITE PLUS NITRATE, TOTAL 1 D = 0.37 -0.000005 DATE +0.000490 FLOW

Predictor	Coef	StDev	T	P
Constant	0.374	1.766	0.21	0.833
DATE	-0.00000501	0.00005549	-0.09	0.928
FLOW	0.0004897	0.0001876	2.61	0.011

S = 0.3061 R-Sq = 9.0% R-Sq(adj) = 6.4%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.63893	0.31946	3.41	0.039
Residual Error	69	6.46367	0.09368		
Total	71	7.10260			

Source	DF	Seq SS
DATE	1	0.00054
FLOW	1	0.63839

Table 36 Regression Statistics for Nitrate plus Nitrite at RM 962

The regression equation is

NITRITE PLUS NITRATE, TOTAL 1 D = - 5.19 +0.000159 DATE - 0.00076 FLOW

13 cases used 10 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	-5.187	4.018	-1.29	0.226
DATE	0.0001586	0.0001176	1.35	0.207
FLOW	-0.000763	0.001267	-0.60	0.561

S = 0.2317 R-Sq = 15.5% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.09828	0.04914	0.92	0.431
Residual Error	10	0.53671	0.05367		
Total	12	0.63499			

Source	DF	Seq SS
DATE	1	0.07885
FLOW	1	0.01943

Table 37 Regression Statistics for Nitrate plus Nitrite at Falls Creek

The regression equation is

NITRITE PLUS NITRATE, TOTAL 1 D = - 0.07 +0.000008 DATE +0.000090 FLOW

Predictor	Coef	StDev	T	P
Constant	-0.066	1.192	-0.06	0.956
DATE	0.00000751	0.00003752	0.20	0.842
FLOW	0.00009006	0.00004844	1.86	0.067

S = 0.2032 R-Sq = 4.8% R-Sq(adj) = 2.1%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.14571	0.07285	1.76	0.179
Residual Error	70	2.88967	0.04128		
Total	72	3.03538			

Source	DF	Seq SS
DATE	1	0.00301
FLOW	1	0.14270

Table 38 ANOVA Statistics for Nitrate plus Nitrite

Analysis of Variance for NITRITE + NITRATE

Source	DF	SS	MS	F	P
STATION	3	1.348	0.449	3.96	0.009
Error	245	27.793	0.113		
Total	248	29.141			

Individual 95% CIs For Mean
Based on Pooled StDev

Level	N	Mean	StDev	CI
15	73	0.1895	0.2053	(-----*-----)
50	13	0.2149	0.2300	(-----*-----)
75	72	0.2783	0.3163	(-----*-----)
150	91	0.3674	0.4349	(-----*-----)

Pooled StDev = 0.3368

0.12 0.24 0.36

* NOTE * N missing = 15

Table 39 Regression Statistics for TKN at RR 1320

The regression equation is

NITROGEN, KJELDAHL, TOTAL, (MG/ = 1.00 -0.000013 DATE +0.000147 FLOW

95 cases used 1 cases contain missing values

Predictor	Coef	StDev	T	P
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Constant	0.995	1.163	0.86	0.394
DATE	-0.00001310	0.00003584	-0.37	0.716
FLOW	0.0001470	0.0002316	0.63	0.527

S = 0.4382 R-Sq = 0.6% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.1028	0.0514	0.27	0.766
Residual Error	92	17.6633	0.1920		
Total	94	17.7661			

Source	DF	Seq SS
DATE	1	0.0255
FLOW	1	0.0773

Table 40 Regression Statistics for TKN at Johnson City

The regression equation is

NITROGEN, KJELDAHL, TOTAL, (MG/ = 8.31 -0.000238 DATE -0.000518 FLOW

Predictor	Coef	StDev	T	P
Constant	8.312	2.687	3.09	0.003
DATE	-0.00023752	0.00008445	-2.81	0.006
FLOW	-0.0005184	0.0002854	-1.82	0.074

S = 0.4657 R-Sq = 14.0% R-Sq(adj) = 11.5%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	2.4437	1.2219	5.63	0.005
Residual Error	69	14.9671	0.2169		
Total	71	17.4108			

Source	DF	Seq SS
DATE	1	1.7282
FLOW	1	0.7155

Table 41 Regression Statistics for TKN at RM 962

The regression equation is

NITROGEN, KJELDAHL, TOTAL, (MG/ = - 9.77 +0.000302 DATE - 0.00025 FLOW

21 cases used 2 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	-9.765	9.506	-1.03	0.318
DATE	0.0003024	0.0002771	1.09	0.289
FLOW	-0.000252	0.001046	-0.24	0.812

S = 0.7903 R-Sq = 6.7% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.8065	0.4032	0.65	0.536
Residual Error	18	11.2432	0.6246		
Total	20	12.0497			

Source	DF	Seq SS
DATE	1	0.7703
FLOW	1	0.0362

Table 42 Regression Statistics for TKN at Falls Creek

The regression equation is

NITROGEN, KJELDAHL, TOTAL, (MG/ = 7.45 -0.000210 DATE -0.000112 FLOW

72 cases used 1 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	7.445	4.660	1.60	0.115
DATE	-0.0002102	0.0001466	-1.43	0.156
FLOW	-0.0001119	0.0001882	-0.59	0.554

S = 0.7891 R-Sq = 3.5% R-Sq(adj) = 0.7%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	1.5390	0.7695	1.24	0.297
Residual Error	69	42.9598	0.6226		
Total	71	44.4988			

Source	DF	Seq SS
DATE	1	1.3188
FLOW	1	0.2203

Table 43 ANOVA Statistics for TKN

Analysis of Variance for NITROGEN TKN					
Source	DF	SS	MS	F	P
STATION	3	1.172	0.391	1.09	0.354
Error	256	91.725	0.358		
Total	259	92.898			

Individual 95% CIs For Mean Based on Pooled StDev					
Level	N	Mean	StDev	-----+-----+-----+-----	
15	72	0.7417	0.7917		(-----*-----)
50	21	0.5678	0.7762	(-----*-----)	
75	72	0.6879	0.4952		(-----*-----)
150	95	0.5905	0.4347	(-----*-----)	

Pooled StDev = 0.5986
 * NOTE * N missing = 4

Table 44 Regression Statistics for Ammonia at RR 1320

The regression equation is
 NITROGEN, AMMONIA, TOTAL (MG/L) = - 0.192 + 0.000008 DATE + 0.000078 FLOW

78 cases used 18 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	-0.1921	0.2863	-0.67	0.504
DATE	0.00000756	0.00000877	0.86	0.392
FLOW	0.00007808	0.00006656	1.17	0.244

S = 0.1035 R-Sq = 2.6% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.02153	0.01076	1.01	0.371
Residual Error	75	0.80314	0.01071		
Total	77	0.82467			

Source	DF	Seq SS
DATE	1	0.00679
FLOW	1	0.01474

Table 45 Regression Statistics for Ammonia at Johnson City

The regression equation is

NITROGEN, AMMONIA, TOTAL (MG/L = 1.19 -0.000035 DATE -0.000055 FLOW

62 cases used 10 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	1.1876	0.7517	1.58	0.119
DATE	-0.00003457	0.00002359	-1.47	0.148
FLOW	-0.00005512	0.00007728	-0.71	0.478

S = 0.1227 R-Sq = 4.3% R-Sq(adj) = 1.1%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.04001	0.02001	1.33	0.272
Residual Error	59	0.88756	0.01504		
Total	61	0.92757			

Source	DF	Seq SS
DATE	1	0.03236
FLOW	1	0.00765

Table 46 Regression Statistics for Ammonia at RM 962

The regression equation is

NITROGEN, AMMONIA, TOTAL (MG/L = - 0.048 +0.000004 DATE -0.000080 FLOW

21 cases used 2 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	-0.0482	0.8562	-0.06	0.956
DATE	0.00000351	0.00002475	0.14	0.889
FLOW	-0.00008036	0.00008899	-0.90	0.378

S = 0.06511 R-Sq = 5.0% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.004045	0.002023	0.48	0.628
Residual Error	18	0.076306	0.004239		
Total	20	0.080351			

Source	DF	Seq SS
DATE	1	0.000588
FLOW	1	0.003457

Table 47 Regression Statistics for Ammonia at Falls Creek

The regression equation is
 NITROGEN, AMMONIA, TOTAL (MG/L = - 0.428 +0.000016 DATE -0.000026 FLOW

Predictor	Coef	StDev	T	P
Constant	-0.4279	0.5710	-0.75	0.456
DATE	0.00001579	0.00001797	0.88	0.382
FLOW	-0.00002633	0.00002320	-1.13	0.260

S = 0.09731 R-Sq = 2.8% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.018826	0.009413	0.99	0.375
Residual Error	70	0.662806	0.009469		
Total	72	0.681633			

Source	DF	Seq SS
DATE	1	0.006632
FLOW	1	0.012194

Table 48 ANOVA Statistics for Ammonia

Analysis of Variance for Ammonia

Source	DF	SS	MS	F	P
STATION	3	0.0235	0.0078	0.69	0.562
Error	213	2.4370	0.0114		
Total	216	2.4605			

Individual 95% CIs For Mean
 Based on Pooled StDev

Level	N	Mean	StDev	CI
15	56	0.0870	0.1048	(-----*-----)
50	21	0.0630	0.0634	(-----*-----)
75	62	0.0794	0.1233	(-----*-----)
150	78	0.0628	0.1035	(-----*-----)

Pooled StDev = 0.1070 0.030 0.060 0.090

* NOTE * N missing = 47

Table 49 Regression Statistics for Total Phosphorus at RR 1320

The regression equation is
 PHOSPHORUS, TOTAL, WET METHOD (= - 0.250 +0.000009 DATE +0.000031 FLOW

Predictor	Coef	StDev	T	P
Constant	-0.2504	0.2083	-1.20	0.233
DATE	0.00000902	0.00000641	1.41	0.163
FLOW	0.00003072	0.00004263	0.72	0.473

S = 0.08067 R-Sq = 2.6% R-Sq(adj) = 0.5%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.016202	0.008101	1.24	0.293
Residual Error	93	0.605264	0.006508		
Total	95	0.621465			

Source	DF	Seq SS
DATE	1	0.012822
FLOW	1	0.003380

Table 50 Regression Statistics for Total Phosphorus at Johnson City

The regression equation is
 PHOSPHORUS, TOTAL, WET METHOD (= 7.23 -0.000221 DATE -0.000349 FLOW

Predictor	Coef	StDev	T	P
Constant	7.228	3.449	2.10	0.040
DATE	-0.0002211	0.0001084	-2.04	0.045
FLOW	-0.0003493	0.0003663	-0.95	0.344

S = 0.5978 R-Sq = 6.9% R-Sq(adj) = 4.2%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	1.8197	0.9099	2.55	0.086
Residual Error	69	24.6560	0.3573		
Total	71	26.4757			

Source	DF	Seq SS
DATE	1	1.4948
FLOW	1	0.3249

Table 51 Regression Statistics for Total Phosphorus at RM 962

The regression equation is

$$\text{PHOSPHORUS, TOTAL, WET METHOD} = 0.20 - 0.000004 \text{ DATE} + 0.000116 \text{ FLOW}$$

Predictor	Coef	StDev	T	P
Constant	0.204	1.579	0.13	0.899
DATE	-0.00000424	0.00004587	-0.09	0.927
FLOW	0.0001161	0.0001937	0.60	0.556

S = 0.1470 R-Sq = 1.9% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.00834	0.00417	0.19	0.826
Residual Error	20	0.43226	0.02161		
Total	22	0.44060			

Source	DF	Seq SS
DATE	1	0.00057
FLOW	1	0.00777

Table 52 Regression Statistics for Total Phosphorus at Falls Creek

The regression equation is

$$\text{PHOSPHORUS, TOTAL, WET METHOD} = 0.202 - 0.000005 \text{ DATE} - 0.000002 \text{ FLOW}$$

Predictor	Coef	StDev	T	P
Constant	0.2022	0.2087	0.97	0.336
DATE	-0.00000530	0.00000657	-0.81	0.422
FLOW	-0.00000189	0.00000848	-0.22	0.824

S = 0.03556 R-Sq = 1.0% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	0.000905	0.000453	0.36	0.700
Residual Error	70	0.088539	0.001265		
Total	72	0.089444			

Source	DF	Seq SS
DATE	1	0.000842
FLOW	1	0.000063

Table 53 ANOVA Statistics for Total Phosphorus

Analysis of Variance for TOTAL PHOSPHOROUS					
Source	DF	SS	MS	F	P
STATION	3	0.589	0.196	1.85	0.139
Error	260	27.627	0.106		
Total	263	28.216			

Individual 95% CIs For Mean Based on Pooled StDev			
Level	N	Mean	StDev
15	73	0.0334	0.0352
50	23	0.0714	0.1415
75	72	0.1482	0.6107
150	96	0.0466	0.0809

Pooled StDev =		0.000	0.080	0.160
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Table 54 Regression Statistics for TOC at RR 1320

The regression equation is
 CARBON, TOTAL ORGANIC (MG/L AS) = 10.3 - 0.000203 DATE - 0.000140 FLOW

95 cases used 1 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	10.329	4.682	2.21	0.030
DATE	-0.0002030	0.0001440	-1.41	0.162
FLOW	-0.0001405	0.0009498	-0.15	0.883

S = 1.794 R-Sq = 2.1% R-Sq(adj) = 0.0%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	6.459	3.230	1.00	0.371
Residual Error	92	296.254	3.220		
Total	94	302.713			

Source	DF	Seq SS
DATE	1	6.389
FLOW	1	0.070

Table 55 Regression Statistics for TOC at Johnson City

The regression equation is

$$\text{CARBON, TOTAL ORGANIC (MG/L AS)} = 39.6 - 0.00111 \text{ DATE} - 0.00131 \text{ FLOW}$$

Predictor	Coef	StDev	T	P
Constant	39.63	13.28	2.98	0.004
DATE	-0.0011138	0.0004173	-2.67	0.009
FLOW	-0.001305	0.001410	-0.93	0.358

S = 2.301 R-Sq = 10.4% R-Sq(adj) = 7.8%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	42.418	21.209	4.00	0.023
Residual Error	69	365.407	5.296		
Total	71	407.824			

Source	DF	Seq SS
DATE	1	37.881
FLOW	1	4.537

Table 56 Regression Statistics for TOC at RM 962

The regression equation is

$$\text{CARBON, TOTAL ORGANIC (MG/L AS)} = - 19.6 + 0.000658 \text{ DATE} + 0.00108 \text{ FLOW}$$

Predictor	Coef	StDev	T	P
Constant	-19.60	14.20	-1.38	0.183
DATE	0.0006577	0.0004124	1.59	0.126
FLOW	0.001077	0.001742	0.62	0.543

S = 1.322 R-Sq = 12.0% R-Sq(adj) = 3.2%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	4.777	2.389	1.37	0.278
Residual Error	20	34.942	1.747		
Total	22	39.719			

Source	DF	Seq SS
DATE	1	4.110
FLOW	1	0.668

Table 57 Regression Statistics for TOC at Falls Creek

The regression equation is
 CARBON, TOTAL ORGANIC (MG/L AS = 17.7 -0.000454 DATE +0.000122 FLOW

Predictor	Coef	StDev	T	P
Constant	17.704	8.792	2.01	0.048
DATE	-0.0004543	0.0002767	-1.64	0.105
FLOW	0.0001225	0.0003572	0.34	0.733

S = 1.498 R-Sq = 3.8% R-Sq(adj) = 1.1%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	6.230	3.115	1.39	0.256
Residual Error	70	157.121	2.245		
Total	72	163.351			

Source	DF	Seq SS
DATE	1	5.966
FLOW	1	0.264

Table 58 ANOVA Statistics for Total Organic Carbon

Analysis of Variance for Total Organic Carbon

Source	DF	SS	MS	F	P
STATION	3	26.22	8.74	2.48	0.062
Error	259	913.61	3.53		
Total	262	939.83			

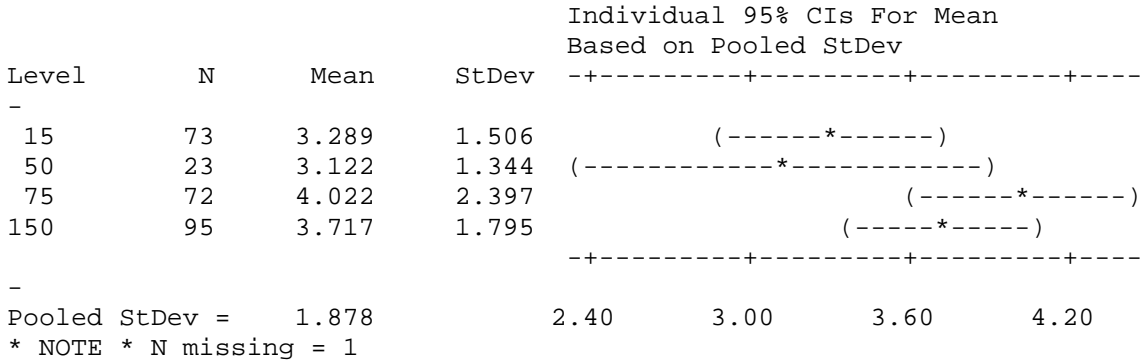


Table 59 Multiple Regression Statistics for Fecal Coliform at RR 1320

The regression equation is

$$\text{FECAL COLIFORM, MEMBR FILTER, M-F} = 1774 - 0.0483 \text{ DATE} + 0.540 \text{ FLOW}$$

95 cases used 1 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	1774	1750	1.01	0.313
DATE	-0.04831	0.05388	-0.90	0.372
FLOW	0.5397	0.3579	1.51	0.135

S = 675.8 R-Sq = 3.2% R-Sq(adj) = 1.1%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	1405229	702615	1.54	0.220
Residual Error	92	42013081	456664		
Total	94	43418311			

Source	DF	Seq SS
DATE	1	366726
FLOW	1	1038503

Table 60 Multiple Regression Statistics for Fecal Coliform at Johnson City

The regression equation is

$$\text{FECAL COLIFORM, MEMBR FILTER, M-F} = 6320 - 0.194 \text{ DATE} + 1.31 \text{ FLOW}$$

Predictor	Coef	StDev	T	P
Constant	6320	6768	0.93	0.354
DATE	-0.1936	0.2127	-0.91	0.366
FLOW	1.3077	0.7189	1.82	0.073

S = 1173 R-Sq = 5.6% R-Sq(adj) = 2.9%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	5667955	2833978	2.06	0.135
Residual Error	69	94937923	1375912		
Total	71	100605879			

Source	DF	Seq SS
DATE	1	1115080
FLOW	1	4552875

Table 61 Multiple Regression Statistics for Fecal Coliform at RM 962

The regression equation is

$$\text{FECAL COLIFORM, MEMBR FILTER, M-F} = 5317 - 0.150 \text{ DATE} + 1.46 \text{ FLOW}$$

22 cases used 1 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	5317	9222	0.58	0.571
DATE	-0.1504	0.2673	-0.56	0.580
FLOW	1.461	1.055	1.38	0.182

S = 792.4 R-Sq = 11.8% R-Sq(adj) = 2.6%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	1601635	800817	1.28	0.302
Residual Error	19	11928952	627840		
Total	21	13530587			

Source	DF	Seq SS
DATE	1	398784
FLOW	1	1202850

Table 62 Multiple Regression Statistics for Fecal Coliform at Falls Creek

The regression equation is

$$\text{FECAL COLIFORM, MEMBR FILTER, M-F} = 3589 - 0.106 \text{ DATE} + 0.598 \text{ FLOW}$$

71 cases used 2 cases contain missing values

Predictor	Coef	StDev	T	P
Constant	3589	10037	0.36	0.722
DATE	-0.1063	0.3158	-0.34	0.737
FLOW	0.5984	0.4040	1.48	0.143

S = 1694 R-Sq = 3.2% R-Sq(adj) = 0.4%

Analysis of Variance

Source	DF	SS	MS	F	P
Regression	2	6518103	3259051	1.14	0.327
Residual Error	68	195198893	2870572		
Total	70	201716996			

Source	DF	Seq SS
DATE	1	221718
FLOW	1	6296384

Table 63 ANOVA Statistics for Fecal Coliform

Source	DF	SS	MS	F	P
STATION	3	144190	48063	0.03	0.992
Error	255	359167039	1408498		
Total	258	359311229			

